Appendix A Reeves Road Closure Detour Assessment

Technical Advice Memorandum

То	Josie Jackson, Andy Gibbard, Julio Marti Herraiz, Ben Burrows	Page	1 of 27
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Subject	AMETI Eastern Busway		
	Stage 2 Reeves Road Closure & Detour Assessment - D	RAFT	
From	Shane Doran		
File/Ref No. EBQ-TA-Z1-TM 0001-01	-	Date	03-Sep-2021

Summary

- Overall, it seems that only a small percentage of traffic is routing along the proposed detour route (probably due to the already congested nature of those intersections).
- As a result, in the inbound (citybound) direction during the AM demand seems to detour via Gossamer Road to Pakuranga Road in the north and Ti Rakau Drive in the south.
- In the PM in the outbound direction, demand seems to return via Pakuranga Road, and via SEART turning right onto Ti Rakau Drive.
- NOTE: Colour-coding of the tables below are in reference to the Do-Minimum scenario with green = improved, amber = similar and red = worse.

William Roberts Road / Reeves Road intersection:

 For the Stage 2 detour this intersection is expected to operate with good LOS in the AM and PM.

Scenario	LC	os	DOS	[v/c]	Average D	elay [sec]
Cochano	AM	PM	АМ	PM	AM	PM
Do-Minimum	-	-	0.68	0.87	8	12
Stage 2 Detour	-	-	0.18	0.39	5	5

William Roberts Road / Pakuranga Road intersection:

The assessment indicates that the Stage 2 detour results in overall intersection performance
that is similar or worse than the Do-Minimum in the AM and PM, but in practical terms the
intersection is already saturated, and the impact of the detour is considered negligible.

Scenario	LC	os	DOS	[v/c]	Average D	elay [sec]
Occitatio	AM	PM	AM	PM	AM	PM
Do-Minimum	-	-	9.46	32.92	387	2260
Stage 2 Detour	-	-	10.96	19.93	443	1011



Pakuranga Road / Ti Rakau Drive intersection:

- Stage 2 intersection performance is LOS D during the AM peak, with the Ti Rakau Drive west left-turn lane expected to operate at LOS C.
- During the PM peak the intersection is expected to perform at LOS F during Stage 2, compared to the LOS E of the Do-Minimum scenario. Demand seems to have increased on Pakuranga Road west (and east), however the west approach is already saturated, causing delays to increase further.
- NOTE: The Ti Rakau Drive / SEART intersection below (Section 3.4) is also expected to experience poor LOS. It is expected that the mitigation measures discussed would remedy that intersection as well as this intersection by drawing demand away from Pakuranga Road eastbound to SEART, then turning right into Ti Rakau Drive. Therefore, mitigation measures are not recommended for the Pakuranga Road / Ti Rakau Drive intersection.

Scenario	LC	os	DOS	[v/c]	Average D	elay [sec]
Scenario	AM	PM	AM	PM	АМ	PM
Do-Minimum	D	Е	0.93	1.01	43	71
Stage 2 Detour	D	F	0.92	1.07	37	99

Ti Rakau Drive / SEART intersection:

- Intersection performance is poor (LOS F) during both the AM and PM peaks for the Do-Min scenario. The Stage 2 AM peak intersection performance is expected to be slightly improved (LOS E), however the PM peak is still poor (LOS F).
- The SEART right-turn lanes into Ti Rakau Drive are expected to operate at LOS F during the PM peak. The increase in demand (due to Reeves Road closure) results in delay increasing from around 50 sec to 215 sec (3.6 min), which would require mitigation.
- Mitigation 1 consists of an additional right-turn lane from SEART to Ti Rakau Drive eastbound and an additional exit lane on Ti Rakau Drive eastbound between SEART and William Roberts Road. Improved intersection performance is expected during both AM and PM peaks. This option is recommended to be discussed with key stakeholders.
- A further refinement of Mitigation 1 was also tested. It consisted of a 105 m short exit lane (AGRD04A (Austroads) - Table 5.5) on Ti Rakau Drive eastbound. Although intersection performance is expected to be similar to Mitigation 1 above, this geometric change would provide insufficient weave distance up to William Roberts Road and is not recommended.
- Mitigation 2 consists of the temporary removal of the pedestrian crossing on the eastern arm of the intersection, reducing phases to 3 and redistributing the greentime. Improved intersection performance is expected during both AM and PM peaks however, large queues are still expected in the SEART right-turn lanes.

Scenario	LO	os	DOS	[v/c]	Average D	elay [sec]
Scenario	AM	PM	AM	PM	AM	PM
Do-Minimum	F	F	0.89	1.15	178	98
Stage 2 Detour	Е	F	0.91	1.13	60	120
Mitigation 1	D	Е	0.89	1.06	53	41
Mitigation 2	D	Е	0.87	0.97	41	77



Ti Rakau Drive / Gossamer Drive intersection:

- The right-turn traffic demand from Gossamer Drive into Ti Rakau Drive is expected to increase by around 160 veh/h. The resultant intersection performance is poor (LOS F) for the Stage 2 AM peak, compared to the LOS E of the Do-Min scenario. This would require mitigation.
- The intersection is expected to experience little change during the Stage 2 PM peak and will remain at LOS D.
- Mitigation 1 consists of the following changes to the northern Gossamer Drive approach: additional short right-turn lane (100 m), converting the short left-turn slip lane to pass through the intersection and providing 150 m stacking space and kerbside short exit lane length increased to 100 m. Intersection performance is expected to be slightly improved, compared to Stage 2 during the AM peak, however still LOS F. Furthermore, the additional control delay now imposed on the Gossamer Drive left-turn (previously a left-turn slip under the Do-Min and Stage 2 scenarios) has resulted in large delays (141 sec) and queues (478 m) in that lane. The PM peak is expected to experience improved performance.
- Mitigation 2 consists of the following changes to the northern Gossamer Drive approach: additional short lane (100 m) for the shared through and right-turn movements, converting the centre lane to a full left-turn lane, converting the short left-turn slip lane to pass through the intersection and providing 150 m stacking space and kerbside short exit lane length increased to 100 m. Improved intersection performance is expected during both the AM and PM peaks. This option is recommended to be discussed with key stakeholders.

Scenario	LC	os	DOS	[v/c]	Average D	elay [sec]
Scenario	AM	PM	AM	PM	AM	PM
Do-Minimum	D	D	1.02	0.90	48	45
Stage 2 Detour	F	D	1.25	0.88	168	43
Mitigation 1	F	D	1.09	0.86	118	38
Mitigation 2	D	D	0.89	0.86	37	37

• The impacts to buses through the project area are expected to be low as the frequencies of the 711 route in the AM inbound and PM outbound directions are only 4 buses/h (1 every 15min).



1.0 Introduction

The proposed construction sequencing of the Eastern Busway, relevant to this Technical Advice Note, are as follows:

Stage 1 - Extension of William Roberts Road south to Ti Rakau Drive as well as completing the Cortina Place link between William Roberts Road and Reeves Road. During this stage, Reeves Road will remain open, providing site access through Cortina Place and William Roberts Road. Figure 1 provides an overview of Stage 1.



Figure 1: Stage 1 overview

Stage 2 - Closure of Reeves Road between Ti Rakau Drive and William Roberts Road. At the completion of Stage 1, William Roberts Road is intended to be a left-in left-out (LILO) only at the intersection with Ti Rakau Drive. Therefore, due to the Reeves Road closure and the LILO arrangement at the William Roberts Road / Ti Rakau Drive intersection, an alternative detour route is proposed for traffic usually traveling along Reeves Road toward the SEART. Figure 2 provides an overview of Stage 2.





Figure 2: Stage 2 overview

The proposed detour will route traffic north along William Roberts Road, west along Pakuranga Road, south along Ti Rakau Drive and finally west along SEART. The purpose of this Technical Advice Note is to assess the proposed detour route.



2.0 Assessment Methodology

AIMSUN:

- Operational Microsimulation Models These models provide information about travel times along different routes within the project area as well as turn movements and traffic demand along roads within the project area.
- The proposed geometric changes of Stage 1 and Stage 2 were updated within the AIMSUN model to determine re-routed traffic demands along roads within the project area.
- The turning movement outputs at intersections from this model were used as inputs into SIDRA models.

SIDRA:

- Intersection models These models are used to determine the performance of intersections
 using traffic movement data from AIMSUN models. The key outputs include degree of
 saturation (DOS), level of service (LOS), delays and queue lengths.
- The proposed geometric changes of Stage 1 and Stage 2 were updated within the SIDRA model and the AIMSUN turning movements were imported.
- The results of this 'Stage 2' scenario were compared to a 'Do Minimum' scenario in order to determine and assess the potential impact of the proposed Stage 2 and associated disruption.



3.0 Lane Performance Summaries

3.1 William Roberts Road / Reeves Road

3.1.1 AM Peak

- NOTE: The Reeves Road west approach to this intersection in the Stage 2 scenario has been closed. For the Stage 2 detour this intersection is expected to operate with good LOS in the AM, the removal of the Reeves Road west approach reduces much of the opposing flows.
- Total intersection demand decreased by around 300 veh/h and the Reeves Road east approach right-turn increased by around 30 veh/h.
- It is clear from the comparison below that Reeves Road westbound traffic demand has decreased and only a small percentage of traffic is routing along the proposed detour north on William Roberts Road. This is a general trend for all intersection assessed and will be detailed further in later sections of this Technical Advice Note.

Table 1: William Roberts Rd / Reeves Rd - Do-Min vs Stage 2 (AM)

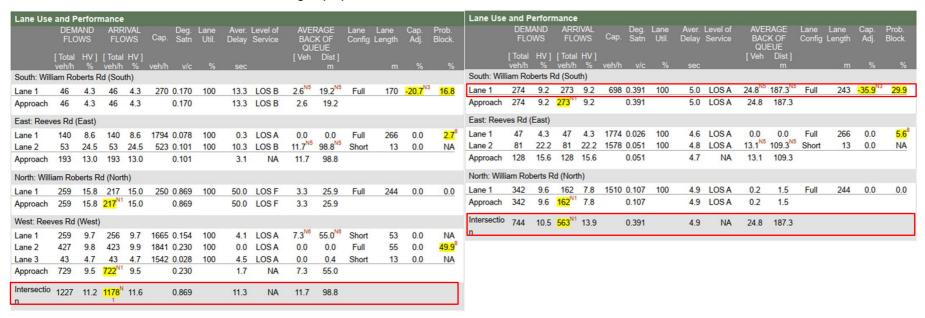
Lane Use	and P	erforr	nance												Lane Use	and P	erforn	nance											
		AND WS	ARR FLC	IVAL WS	Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane I Config L		Cap. Adj.	Prob. Block.		DEM FLC		ARRI FLO		Deg. Cap. Satn	Lane Util.	Aver. Delay	Level of Service	BAC	RAGE K OF EUE	Lane Config	Lane Length		Prob. Block.
	[Total veh/h				veh/h v/c		sec		[Veh							[Total veh/h	HV]	[Total veh/h	HV]	veh/h v/c		sec		[Veh					
South: Willi	iam Ro	berts R	d (Sou	th)											South: Will	iam Ro	berts R	d (Sout	th)										
Lane 1	39	2.6	39	2.6	337 0.116	100	10.9	LOS B	0.1	0.9	Full	170	-20.4 ^{N7}	0.0	Lane 1	136	7.4	135	7.4	745 0.182	100	5.9	LOSA	0.6 ^{N5}	4.5 ^{N5}	Full	243	-19.0 ^{N3}	11.8
Approach	39	2.6	39	2.6	0.116		10.9	LOS B	0.1	0.9					Approach	136	7.4	135 ^{N1}	7.4	0.182		5.9	LOSA	0.6	4.5				
East: Reev	es Rd (East)													East: Reev	res Rd (East)												
Lane 1	375	6.1	375	6.1	1831 0.205	100	0.2	LOSA	0.0	0.0	Full	266	0.0	0.0	Lane 1	215	5.6	215	5.6	1758 0.122	100	4.6	LOSA	0.0	0.0	Full	266	0.0	0.0
Lane 2	103	11.7	103	11.7	562 0.183	100	5.5	LOSA	0.2	1.2	Short	13	-49.9 ^N	NA.	Lane 2	134	9.0	134	9.0	1718 0.078	100	4.7	LOSA	2.6 ^{N5}	19.4 ^{N5}	Short	13	0.0	NA
Approach	478	7.3	478	7.3	0.205		1.3	NA	0.2	1.2					Approach	349	6.9	349	6.9	0.122		4.6	NA	2.6	19.4				
North: Willi	am Rob	erts R	d (Nort	h)											North: Will	iam Rob	erts R	d (North	h)										
Lane 1	283	5.7	235	4.2	344 0.683	100	24.2	LOSC	2.0	14.6	Full	244	0.0	0.0	Lane 1	212	12.3	89	7.7	1216 0.073	100	5.3	LOSA	0.1	0.9	Full	244	0.0	0.0
Approach	283	5.7	235 ^{N1}	4.2	0.683		24.2	LOSC	2.0	14.6					Approach	212	12.3	89 ^{N1}	7.7	0.073		5.3	LOSA	0.1	0.9				
West: Reev	ves Rd	(West)													Intersectio	697	8.6	573 ^{N1}	10.5	0.182		5.0	NA	2.6	19.4				
Lane 1	41	9.8	40	9.9	832 0.048	100	4.1	LOSA	0.0	0.0	Short	53	-49.9 ^{N7}	NA.	n		10111	-				- 111	11						
Lane 2	143	12.6	139	12.8	1803 0.077	100	0.0	LOSA	0.0	0.0	Full	55	0.0	0.0															
Lane 3	11	0.0	11	0.0	1238 0.009	100	5.4	LOSA	0.0	0.1	Short	13	0.0	NA															
Approach	195	11.3	190 ^{N1}	11.4	0.077		1.2	NA	0.0	0.1																			
Intersectio n	995	7.4	942 ^{N1}	7.9	0.683		7.4	NA	2.0	14.6																			



3.1.2 PM Peak

- The intersection is expected to operate with good LOS in PM with the Stage 2 detour.
- Total intersection demand decreased by around 500 veh/h and the Reeves Road east approach right-turn increased by around 30 veh/h. The William Roberts Road south approach demand increased by around 230 veh/h, indicating an increase in vehicles routing from Ti Rakau Drive at the new LILO intersection further south.

Table 2: William Roberts Rd / Reeves Rd - Do-Min vs Stage 2 (PM)





3.2 William Roberts Road / Pakuranga Road

3.2.1 AM Peak

- The Do-Min scenario performance is poor, specifically the shared left and right-turn lane out of William Roberts Road and the Pakuranga Road west right-turn during the AM.
- The results below indicate that the Stage 2 detour results in overall intersection performance that is worse than the Do-Min in the AM, but in practical terms the intersection is saturated, and the impact of the detour is considered negligible.

Table 3: William Roberts Rd / Pakuranga Rd - Do-Min vs Stage 2 (AM)

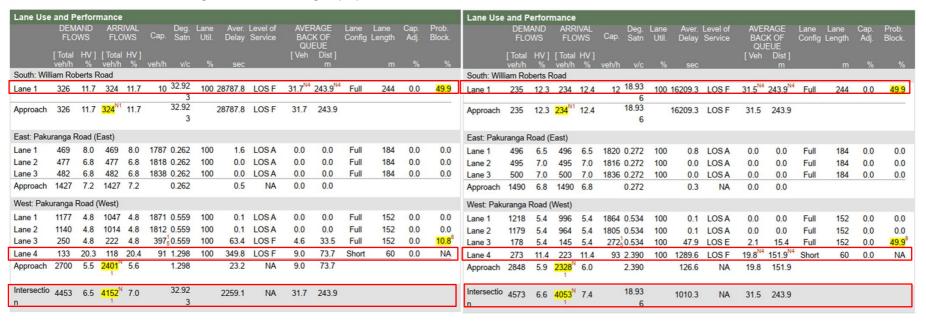
Lane Use	and P	erfori	mance												Lane Use	and P	erforn	nance	ķ.										
	DEM FLC		ARR FLO		Deg. Cap. Satn	Lane Util.		Level of Service	AVEF BACI QUE	K OF	Lane Config	Lane Length	Cap. Adj.	Prob. Block.		DEM FLC	AND WS	ARR FLC	IVAL WS	Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config L	Lane Length	Cap. Adj.	Prob. Block.
			[Total veh/h		veh/h v/c		sec		[Veh	Dist] m						[Total veh/h				veh/h v/c		sec		[Veh	Dist]				%
South: Will	iam Ro	berts F	Road												South: Will	iam Ro	berts R	load											
Lane 1	147	9.5	146	9.6	15 9.462	100	7695.3	LOSF	32.2 ^{N4}	243.9 ^{N4}	Full	244	0.0	49.9	Lane 1	159	8.2	159	8.2	32 4.904	100	3578.9	LOSF	32.6 ^{N4}	243.9 ^{N4}	Full	244	0.0	49.9
Approach	147	9.5	146 ^{N1}	9.6	9.462		7695.3	LOSF	32.2	243.9					Approach	159	8.2	159	8.2	4.904		3578.9	LOSF	32.6	243.9				
East: Paku	ranga F	Road (East)												East: Paku	ranga F	Road (E	ast)											
Lane 1	991	5.3	991	5.3	1824 0.543	100	1.4	LOSA	0.0	0.0	Full	184	0.0	0.0	Lane 1	766	5.7	766	5.7	1832 0.418	100	0.6	LOSA	0.0	0.0	Full	184	0.0	0.0
Lane 2	825	5.7	825	5.7	1519 0.543	100	0.1	LOSA	0.0	0.0	Full	184	-17.0 ^{N7}	0.0	Lane 2	765	5.9	765	5.9	1829 0.418	100	0.1	LOSA	0.0	0.0	Full	184	0.0	0.0
Lane 3	866	5.7	866	5.7	1593 0.543	100	0.1	LOSA	0.0	0.0	Full	184	-13.9 ^{N7}	0.0	Lane 3	773	5.9	773	5.9	1849 0.418	100	0.1	LOSA	0.0	0.0	Full	184	0.0	0.0
Approach	2682	5.6	2682	5.6	0.543		0.6	NA	0.0	0.0					Approach	2305	5.8	2305	5.8	0.418		0.2	NA	0.0	0.0				
West: Paki	uranga	Road (West)												West: Paku	ıranga	Road (West)											
Lane 1	526	6.8	509	6.9	1847 0.276	100	0.0	LOSA	0.0	0.0	Full	152	0.0	0.0	Lane 1	724	7.2	710	7.3	1842 0.386	100	0.0	LOSA	0.0	0.0	Full	152	0.0	0.0
Lane 2	509	6.8	493	6.9	1788 0.276	100	0.0	LOSA	0.0	0.0	Full	152	0.0	0.0	Lane 2	701	7.2	688	7.3	1784 0.386	100	0.0	LOSA	0.0	0.0	Full	152	0.0	0.0
Lane 3	11	6.8	11	6.9	39 ₅ 0.276	100	498.4	LOSF	1.6	11.5	Full	152	0.0	48.6 ⁸	Lane 3	20	7.2	20	7.3	51 ₅ 0.386	100	268.2	LOSF	1.5	11.5	Full	152	0.0	49.9 ⁸
Lane 4	54	13.0	52	13.2	6 8.723	100	7175.8	LOSF	19.3	150.2	Short	60	0.0	NA	Lane 4	135	15.6	133	15.7	12 10.96	100	9061.0	LOSF	19.1 ^{N4}	151.9 ^{N4}	Short	60	0.0	NA
Approach	1100	7.1	1065 ^N	7.2	8.723		357.6	NA	19.3	150.2					Approach	1580	7.9	1550 ^N	8.0	10.96		778.4	NA	19.1	151.9				
Intersectio	2000	0.0	naanN	0.0	0.400		200.0		20.0	040.0					1,1000			1	-10	2									
n	3929	6.2	3893 ^N	6.2	9.462		386.9	NA	32.2	243.9					Intersectio	4044	6.7	4014 ^N	6.8	10.96		442.4	NA	32.6	243.9				
															ln	- 11		1	******	2									



3.2.2 PM Peak

- Similar to the AM, the Do-Min scenario performance is poor, specifically the shared left and right-turn lane out of William Roberts Road and the Pakuranga Road west right-turn during the PM.
- In practical terms the intersection is already over-saturated in the Do-Min PM peak, and the impact of the detour is considered negligible.
- During the Stage 2 detour the Pakuranga Road west right-turn lane is expected to perform worse than the Do-Min during the PM, with high delay and LOS F.

Table 4: William Roberts Rd / Pakuranga Rd - Do-Min vs Stage 2 (PM)





3.3 Pakuranga Road / Ti Rakau Drive

3.3.1 AM Peak

- The Stage 2 detour intersection performance is expected to be only slightly improved during the AM peak compared to the Do-Min scenario.
- The Pakuranga Road east left-turn is expected to operate at LOS C.
- Total intersection demand increased by around 300 veh/h, with increases on Ti Rakau Drive south approach and the Pakuranga Road west approach.

Table 5: Pakuranga Rd / Ti Rakau Dr - Do-Min vs Stage 2 (AM)

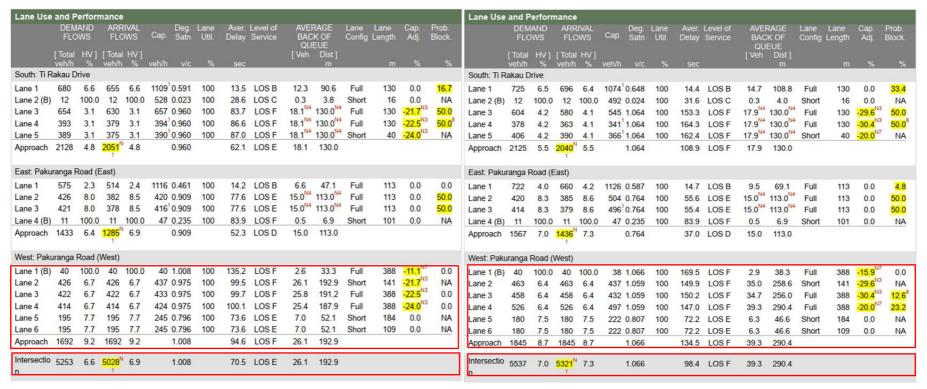
Lane Use	and F	erforn	nance												Lane Use	and F	erforr	nance											
	FLC		ARR FLO	WS	Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config	Lane Length	Cap. Adj.	Prob. Block.			MAND DWS	ARR FLC	RIVAL DWS	Cap. Deg. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	[Total veh/h	HV]			veh/h v/c	%	sec		[Veh	Dist] m		m	%	%		[Total veh/h	HV]	[Total veh/h		veh/h v/c		sec		[Veh	Dist] m				
South: Ti R	akau D	rive													South: Ti R	akau [Drive												
Lane 1	583	7.5	509	7.8	759 ¹ 0.671	100	29.8	LOSC	14.4	107.9	Full	130	0.0	32.6	Lane 1	701	9.1	635	9.3	841 0.754	100	24.4	LOSC	17.2 ^{N4}	130.0 ^{N4}	Full	130	0.0	50.0
Lane 2 (B)	14	100.0	14	100.0	326 0.043	100	48.0	LOS D	0.5	6.0	Short	16	0.0	NA	Lane 2 (B)	16	100.0	16	100.0	348 0.046	100	45.7	LOS D	0.5	6.7	Short	16	0.0	NA
Lane 3	166	4.6	145	4.8	502 0.288	100	50.5	LOSD	5.1	37.4	Full	130	0.0	0.0	Lane 3	225	4.4	203	4.5	538 0.378	100	49.6	LOS D	7.3	52.9	Full	130	0.0	0.0
Lane 4	166	4.6	145	4.8	502 0.288	100	50.5	LOS D	5.1	37.4	Full	130	0.0	0.0	Lane 4	203	4.4	184	4.5	487 0.378	100	49.0	LOS D	6.5	47.2	Full	130	0.0	0.0
Lane 5	166	4.6	145	4.8	502 0.288	100		LOSD	5.1	37.4	Short	40	0.0	NA	Lane 5	203	4.4	184	4.5	487 0.378	100	49.0	LOSD	6.5	47.2	Short	40	0.0	NA
Approach	1094	7.4	957 ^{N1}	7.8	0.671		39.5	LOS D	14.4	107.9					Approach	1348	8.0	1221 ^N	8.2	0.754		36.3	LOS D	17.2	130.0				
East: Paku	ranga F	Road (E	ast)																										
Lane 1	1019	4.2	974	4.3	1113 0.875	100	20.0	LOS B	15.6 ^{N4}	113.0 ^{N4}	Full	113	0.0	50.0	East: Paku	-	Company A	1000						Na	N/A				
Lane 2	757	5.8	724	6.0	781 0.927	100	62.7	LOS E		113.0 ^{N*}	Full	113	0.0	50.0	Lane 1	1062			W. States	1097 0.915	100	1000	LOS C		113.0 ^{N4}	Full	113	0.0	50.0
Lane 3	740	5.8	707	6.0	763 ¹ 0.927	100	62.6	LOSE	15.4 ^{N4}	113.0 ^{N4}	Full	113	0.0	50.0	Lane 2	592	5.3	559	5.4	735 0.761	100	42.8			113.0	Full	113	0.0	50.0
Lane 4 (B)	19	100.0	19	100.0	47 0.406	100	85.2	LOSF	0.9	12.2	Short	101	0.0	NA	Lane 3	574	5.3	543	5.4	713 0.761	100	42.3			113.0 ^{N4}		113	0.0	50.0
Approach	2535	5.9	2424 ^N	6.0	0.927		45.7	LOSD	15.6	113.0					Lane 4 (B) Approach	19 2247	100.0 5.9	19 2125 ^N	6.0	47 0.406 0.915	100	85.2 36.8	LOS D	0.9 15.5	12.2 113.0	Short	101	0.0	NA
West: Paku	ıranga	Road (\	Nest)															1											
Lane 1 (B)	24	100.0	24	100.0	44 0.541	100	90.8	LOSF	1.2	15.6	Full	388	0.0	0.0	West: Paku	ıranga	Road (West)											
Lane 2	201	7.8	201	7.8	893 0.225	100	24.1	LOSC	5.2	39.0	Short	141	0.0	NA	Lane 1 (B)	24	100.0	24	100.0	35 0.690	100	95.0	LOS F	1.3	16.3	Full	388	-21.6 ^N	0.0
Lane 3	201	7.8	201	7.8	893 0.225	100	24.1	LOSC	5.2	39.0	Full	388	0.0	0.0	Lane 2	319	9.4	319	9.4	848 0.376	100	28.1	LOSC	9.3	70.2	Short	141	0.0	NA
Lane 4	201	7.8	201	7.8	893 0.225	100	24.1	LOSC	5.2	39.0	Full	388	0.0	0.0	Lane 3	319	9.4	319	9.4	848 0.376	100	28.1	LOSC	9.3	70.2	Full	388	0.0	0.0
Lane 5	177	13.6	177	13.6	226 0.782	100	62.4	LOSE	6.0	47.2	Short	184	0.0	NA	Lane 4	319	9.4	319	9.4	848 0.376	100	28.1	LOSC	9.3	70.2	Full	388	0.0	0.0
Lane 6	177	13.6	177	13.6	226 0.782	100	62.4	LOSE	6.0	47.2	Short	109	0.0	NA	Lane 5	187	13.9	187	13.9	236 0.792	100	64.6		6.6	51.4	Short	184	0.0	NA
Approach	981	12.1	981	12.1	0.782		39.5	LOSD	6.0	47.2					Lane 6	130	13.9		13.9	164 0.792 0.792	100	67.5		9.3	37.0 70.2	Short	109	-30.6	NA
Intersectio	4610	76	4361 ^N	8.0	0.927		429	LOS D	15.6	113.0					Approach	1297	12.2	1297	12.2	0.792		38.6	LOS D	9.3	70.2				
n	4010	7.0	1	0.0	0.521		72.3	2000	13.0	110.0					Intersectio	4892	8.1	4643 ^N	8.6	0.915		37.2	LOSD	17.2	130.0				



3.3.2 PM Peak

- During the PM peak the intersection is expected to perform at LOS F during Stage 2, compared to the LOS E of the Do-Min scenario.
- Demand seems to have increased on Pakuranga Road west (and east), however the west approach is already saturated, causing delays to increase further.
- NOTE: The Ti Rakau Drive / SEART intersection (Section 3.4 below) is also expected to experience poor LOS. It is expected that the mitigation
 measures discussed in Section 3.4.3 would remedy that intersection as well as this intersection by drawing demand away from Pakuranga Road
 eastbound to SEART, then turning right into Ti Rakau Drive. Therefore, mitigation measures are not recommended for the Pakuranga Road / Ti
 Rakau Drive intersection.

Table 6: Pakuranga Rd / Ti Rakau Dr - Do-Min vs Stage 2 (PM)



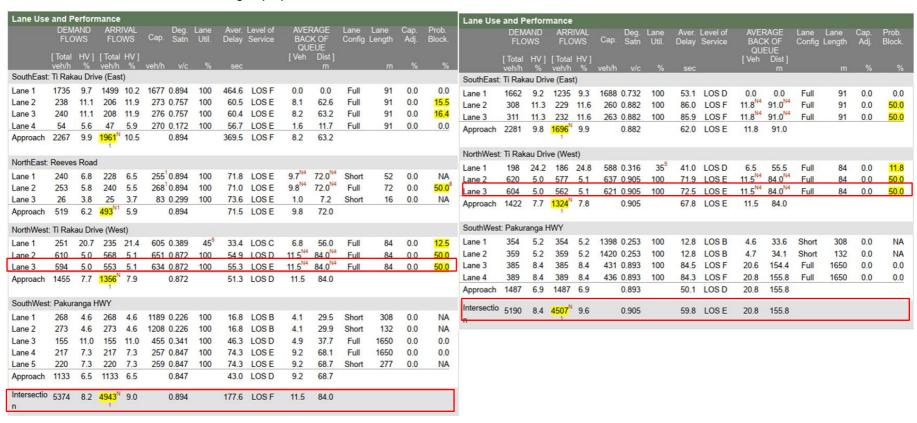


3.4 Ti Rakau Drive / SEART (Pakuranga HWY)

3.4.1 AM Peak

- Do-Min scenario performance is poor, LOS F in the AM peak.
- Stage 2 intersection performance is expected to be improved with lower delay but poorer DOS, LOS is E in the AM peak.
- The Ti Rakau Drive west approach right-turn into SEART is expected to operate at LOS E during the AM.

Table 7: Ti Rakau Dr / SEART - Do-Min vs Stage 2 (AM)

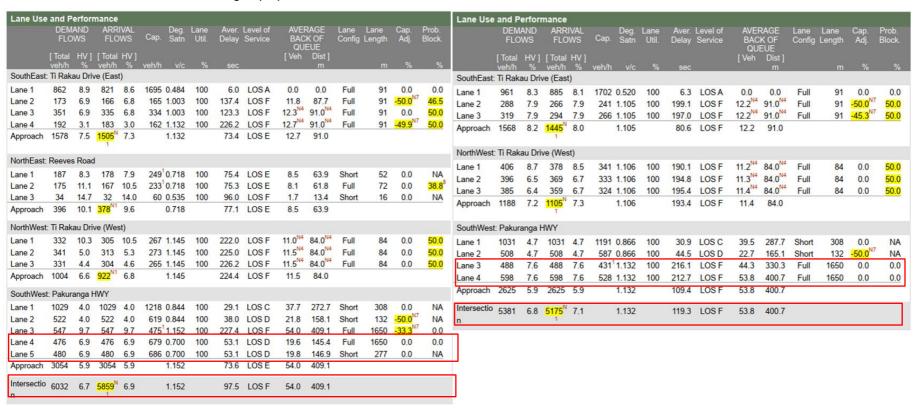




3.4.2 PM Peak

- Similar to the AM, the Do-Min intersection performance is poor (LOS F). The intersection performance is also expected to be poor during the Stage 2 detour.
- The SEART right-turn lanes into Ti Rakau Drive are expected to operate at LOS F during the PM. The increase in demand (due to Reeves Road closure) results in delay increasing from around 50 sec to 215 sec (3.6 min), which would require mitigation.

Table 8: Ti Rakau Dr / SEART - Do-Min vs Stage 2 (PM)





3.4.3 Mitigation 1

- Mitigation 1, to remedy the Ti Rakau Drive / SEART intersection during the PM peak, includes converting the through lane from SEART to Reeves Road to a right-turn lane (note Reeves Road is closed during Stage 2), thus providing a 3rd lane for this turning movement onto Ti Rakau Drive eastbound.
- In addition, a 3rd exit lane is proposed on Ti Rakau Drive eastbound between the SEART intersection and the newly completed William Roberts Road intersection. The proposed layout is shown below.
- As stated in **Section 3.3** above, it is expected that the Pakuranga Road / Ti Rakau Drive intersection performance during the PM peak would also be improved as demand would be drawn away from that intersection to the Ti Rakau Drive / SEART intersection, which could have more spare capacity.

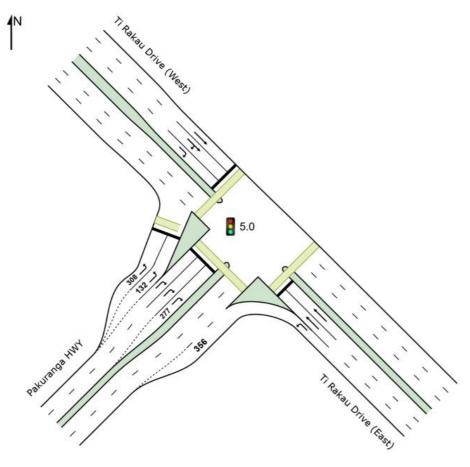
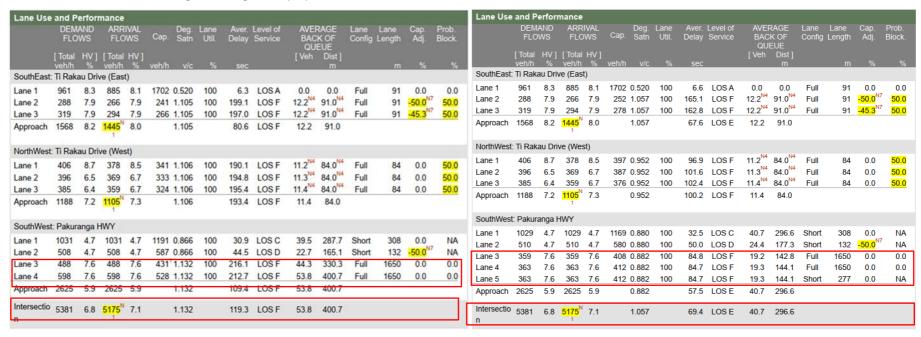


Figure 3: Ti Rakau Dr / SEART - Mitigation 1 proposed layout



- The Stage 2 intersection performance (without mitigation) is expected to be poor (LOS F), especially in the SEART right-turn lanes into Ti Rakau Drive where delay increases from around 50 sec in the Do-Min scenario, to 215 sec (3.6 min).
- With the measures in place proposed under Mitigation 1, intersection performance during the detour is expected to improve to LOS E, with an average delay of 70 sec. This is also an improvement compared to the Do-Min scenario (LOS F and delay of 98 sec)
- Delay and queue lengths in the SEART right-turn lanes are improved under Mitigation 1 (delay of 85 sec and average queue length of 145 m), compared to the Stage 2 detour.

Table 9: Ti Rakau Dr / SEART - Stage 2 vs Mitigation 1 (PM)



- A further refinement of Mitigation 1 was also tested. It consisted of a 105 m short exit lane (AGRD04A (Austroads) Table 5.5) on Ti Rakau Drive eastbound. Although intersection performance is expected to be similar to Mitigation 1 above, this geometric change would provide insufficient weave distance up to William Roberts Road and is not recommended.
- The construction costs of and effort to implement the measures proposed under Mitigation 1 are potentially considerable. Mitigation 1 could require the buildout of the southern side of Ti Rakau Drive and the relocation of the centreline in order to provide the additional exit lane along Ti Rakau Drive eastbound.
- Therefore, alternative measures are proposed under Mitigation 2.



3.4.4 Mitigation 2

- Mitigation 2 includes the temporary removal of the pedestrian crossing on the eastern arm of the Ti Rakau Drive / SEART intersection, thus
 reducing signal phasing down to 3 phases and allowing for more greentime to be allocated to the SEART right-turn movements into Ti Rakau
 Drive eastbound during the PM peak.
- It is noted that pedestrian amenity at this intersection would be temporarily reduced, however the intersection would still provide the ability to cross Ti Rakau Drive from all directions, albeit with some increase in travel time and distance. The pedestrian crossing would be reinstated in the final build-out of the intersection.
- The proposed layout is shown below.

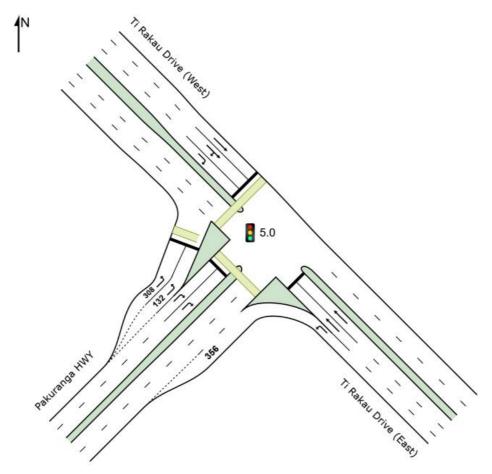


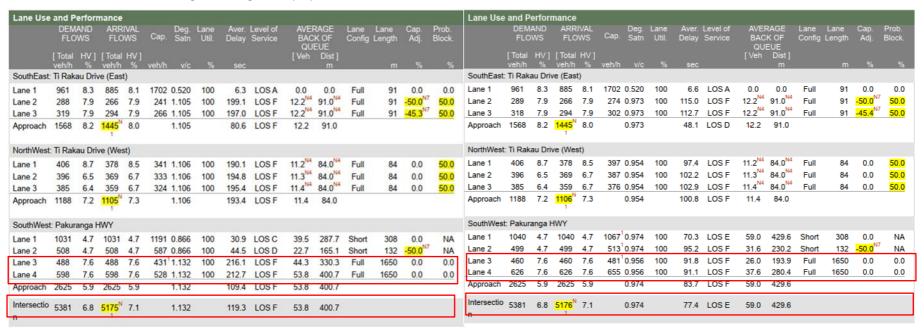
Figure 4: Ti Rakau Dr / SEART - Mitigation 2 proposed layout





- Again, the Stage 2 intersection performance (without mitigation) is expected to be poor (LOS F), especially in the SEART right-turn lanes into Ti
 Rakau Drive where delay increases from around 50 sec in the Do-Min scenario, to 215 sec (3.6 min).
- With the measures in place proposed under Mitigation 2, intersection performance during the detour is expected to improve to LOS E, with an average delay of 78 sec. This is also an improvement compared to the Do-Min scenario (LOS F and delay of 98 sec).
- Delay and queue lengths in the SEART right-turn lanes are improved under Mitigation 2 (delay of 92 sec and average queue length of 280 m), compared to the Stage 2 detour. However, these are still larger than the Do-Min scenario.

Table 10: Ti Rakau Dr / SEART - Stage 2 vs Mitigation 2 (PM)



• The construction costs of and effort to implement the measures proposed under Mitigation 2 are considered more economically viable, compared to Mitigation 1. The measure would include minor road marking removal, signal head amendments and signal phasing adjustments. However, large queues are still expected in the SEART right-turn lanes. Therefore, it is recommended that Mitigation 1 be discussed with key stakeholders.



3.5 Ti Rakau Drive / Gossamer Drive

3.5.1 AM Peak

- The relatively low increase in traffic demand on the right-turn from Reeves Road to William Roberts (**Section 3.1**) indicated that traffic would be diverting to other links in the network. **Section 4.0** below provides further detail to this, but in summary, traffic demand is expected to detour from Gossamer Drive, north to Pakuranga Road and south Ti Rakau Drive.
- As a result, more demand would be experienced on the turning movements into Pakuranga Road and Ti Rakau Drive at these intersections. The Gossamer Drive left-turn into Pakuranga Road is expected to experience an increase of around 40 veh/h only, and was therefore not analysed.
- However, the right-turn from Gossamer Drive into Ti Rakau Drive is expected to increase by around 160 veh/h. The resultant intersection performance is poor (LOS F) for the Stage 2 AM peak, compared to the LOS E of the Do-Min scenario. This may require mitigation.

Table 11: Ti Rakau Dr / Gossamer Dr - Do-Min vs Stage 2 (AM)

Lane Use	and P	erfor	nance		20					NS 2900S					Lane Use	and P	erforn	nance											
	DEM FLC		ARR FLC		Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config I		Cap. Adj.	Prob. Block.	Construction Affects a limite	DEM		ARR FLC		Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config	Lane Length	Cap. Adj.	Prob. Block.
			[Total veh/h		veh/h v/c		sec		[Veh							[Total veh/h				veh/h v/c		sec			Dist]				
South: Fren	nantle	Place													South: Fren	nantle l													
Lane 1 Lane 2	25 31	4.0	25 31	4.0 3.2	82 0.305 85 0.363	100	V. 100 T. 100	LOS E	1.0	7.3 9.0	Short	26 285	0.0	NA 0.0	Lane 1 Lane 2	23 28	8.7 7.1	23 28	8.7 7.1	63 0.367 65 0.429	100 100	94.2 92.7	LOS F	1.2	8.9 10.8	Short	26 285	0.0	NA 0.0
Approach	56	3.6	56	3.6	0.363		72.7	LOSE	1.3	9.0					Approach	51	7.8	51	7.8	0.429		93.4	LOSF	1.4	10.8			0.0	
East: Ti Ra	kau Dri	ive (Ea	st)												East: Ti Ra	kau Dri	ve (Ea	st)											
Lane 1 Lane 2	866 827	11.2 11.3	866 827	11.2	1010 0.857 964 ¹ 0.857	100		LOS C	29.7 28.1	227.8 215.6	Full Full	636 636	0.0	0.0	Lane 1	832 792	10.3	832 792	10.3	680 1.224 646 1.224	100	272.4 277.1	LOSF	85.1 83.4	648.8 635.9	Full Full	636	0.0	51.9 50.0
Lane 3	47	8.4	47	8.4	318 0.147	236		LOSC	0.9	6.8	Short	150	0.0	NA	Lane 2 Lane 3	65	7.8	65	7.8	262 0.247	23 ⁶	700000	LOS D	1.8	13.7	Short	636 150	0.0	NA
Lane 4	203 1943	8.4	203 1943	8.4	318 0.639 0.857	100		LOS C	4.4 29.7	33.2 227.8	Short	103	0.0	NA	Lane 4	282 1971	7.8	282 1971	7.8	262 1.075 1.224	100	137.8	LOS F	16.5 85.1	123.4 648.8	Short	103	0.0	NA
Approach			1943	10.9	0.057		29.1	LUSC	29.7	221.0					Approach			1971	9.9	1.224		241.4	LUST	05.1	040.0				
North: Gos					198										North: Gos		Drive												
ane 1								LOSE		437.3		30	0.0	NA	Lane 1	960	9.0	960		1066 0.901	100			30.7	231.8		30	0.0	NA
Lane 2 Approach	100	8.0 7.5	100	8.0	268 0.374 1.020	100		LOSE	3.6 58.7	26.6 437.3	Full	1010	0.0	0.0	Lane 2 Approach	264 1224	6.4 8.4	264 1224	6.4	212 1.246 1.246	100	319.1	LOSF	29.7	219.6	Full	1010	0.0	0.0
Approach	1155	1.5	1155	1.5	1.020		05.0	LUST	30.7	431.3					1.				0.4	1.240		51.5	LOGI	30.1	231.0				
West: Ti Ra	kau Dr	rive (W	est)												West: Ti Ra	kau Dr	ive (W	est)											
Lane 1	428	10.2	360	10.4	701 0.514	100	33.7	LOSC	10.5	80.0	Full	479	0.0	0.0	Lane 1	523	11.8			451 0.958	100	98.1	LOSF	26.1	200.6	Full	479	0.0	0.0
Lane 2	409	10.4	343	10.6	669 0.514	100	A STREET STREET	LOSC	10.1	77.2	Full	479	0.0	0.0	Lane 2	500	11.9			431 0.958	100	96.7	LOSF	25.1	193.1	Full	479	0.0	0.0
Lane 3	10	10.0	8	9.9	90 0.093	100		LOSE	0.3	2.5	Short	27	0.0	NA	Lane 3	10	10.0	8 05.4N1	9.6	102 0.081	100	85.7	LOSF	0.4	3.0	Short	27	0.0	NA
Approach	847	10.3	712 ^{N1}	10.5	0.514		33.6	LOSC	10.5	80.0					Approach	1033	11.8	854 ^{N1}	11.7	0.958		97.3	LOSF	26.1	200.6				
Intersectio	3999	9.7	3864 ^N	10.0	1.020		47.5	LOS D	58.7	437.3					Intersectio n	4279	9.9	4100 ^N	10.4	1.246		167.6	LOSF	85.1	648.8				



3.5.2 PM Peak

- The intersection is expected to experience little change during the Stage 2 PM peak and will remain at LOS D.
- It is noted that an equivalent in traffic demand returning to this intersection, compared to AM peak, does not occur as expected in the Ti Rakau Drive west left-turn. This could be due to the completion of the William Roberts Road link to Ti Rakau Drive.

Table 12: Ti Rakau Dr / Gossamer Dr - Do-Min vs Stage 2 (PM)

Lane Use	and P	erforr	mance												Lane Use	and P	erforr	nance											
	DEM. FLO	WS	ARR FLO	ws	Deg. Cap. Satn	Lane Util.		Level of Service	BAC QU	RAGE K OF EUE	Lane Config I	Lane Length	Cap. Adj.	Prob. Block.		DEM FLO	WS	ARR FLO	WS		Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config	Lane Length	Cap. Adj.	Prob. Block.
			[Total veh/h		veh/h v/c		sec		[Veh	Dist] m						[Total veh/h		[Total veh/h		veh/h v/c		sec		[Veh	Dist] m				%
South: Fre	mantle l	Place													South: Free	mantle l	Place												
Lane 1	12	0.0	12	0.0	64 0.187	100	95.8	LOSF	0.6	4.4	Short	26	0.0	NA	Lane 1	10	0.0	10	0.0	65 0.155	100	94.9	LOSF	0.5	3.6	Short	26	0.0	NA
Lane 2	27	3.7	27	3.7	65 0.418	100	95.8	LOSF	1.4	10.4	Full	285	0.0	0.0	Lane 2	27	3.7	27	3.7	65 0.416	100	95.2	LOSF	1.4	10.4	Full	285	0.0	0.0
Approach	39	2.6	39	2.6	0.418		95.8	LOSF	1.4	10.4					Approach	37	2.7	37	2.7	0.416		95.1	LOSF	1.4	10.4				
East: Ti Ra	kau Dri	ve (Ea	st)												East: Ti Ra	kau Dri	ve (Ea	st)											
Lane 1	863	6.9	863	6.9	1037 0.832	100	32.6	LOSC	35.4	262.7	Full	636	0.0	0.0	Lane 1	888	6.9	888	6.9	1032 0.860	100	33.7	LOSC	37.5	277.7	Full	636	0.0	0.0
Lane 2	779	7.0	779	7.0	936 0.832	100	30.4	LOSC	30.0	223.0	Full	636	0.0	0.0	Lane 2	806	7.0	806	7.0	937 0.860	100	32.3	LOSC	32.3	239.9	Full	636	0.0	0.0
Lane 3	113	8.9	113	8.9	553 0.205	23 ⁶	28.4	LOSC	2.5	18.9	Short	150	0.0	NA	Lane 3	110	8.5	110	8.5	548 0.201	236	28.5	LOSC	2.4	18.2	Short	150	0.0	NA
Lane 4	494	8.9	494	8.9	553 0.893	100	48.2	LOS D	17.4	130.7	Short	103	0.0	NA	Lane 4	480	8.5	480	8.5	548 0.876	100	45.3	LOS D	16.1	121.1	Short	103	0.0	NA
Approach	2249	7.5	2249	7.5	0.893		35.1	LOSD	35.4	262.7					Approach	2284	7.4	2284	7.4	0.876		35.4	LOSD	37.5	277.7				
North: Gos	samer [Drive													North: Gos	samer (Drive												
Lane 1	475	17.3	475	17.3	911 0.522	100	18.4	LOS B	11.3	91.1	Short	30	0.0	NA	Lane 1	455	16.0	455	16.0	909 0.501	100	17.2	LOS B	10.3	81.8	Short	30	0.0	NA
Lane 2	41	4.9	41	4.9	241 0.170	100	74.3	LOSE	1.9	13.6	Full	1010	0.0	0.0	Lane 2	51	5.9	51	5.9	240 0.212	100	74.6	LOSE	2.3	17.1	Full	1010	0.0	0.0
Approach	516	16.3	516	16.3	0.522		22.8	LOSC	11.3	91.1					Approach	506	15.0	506	15.0	0.501		23.0	LOSC	10.3	81.8				
West: Ti Ra	akau Dr	ive (W	est)												West: Ti Ra	akau Dr	ive (W	est)											
Lane 1	587	5.2	563	5.2	629 0.895	100	73.2	LOSE	29.7	217.3	Full	479	0.0	0.0	Lane 1	603	4.9	560	4.8	639 0.876	100	66.0	LOSE	28.0	203.8	Full	479	0.0	0.0
Lane 2	554	5.5	532	5.5	594 0.895	100	68.8	LOSE	28.3	207.4	Full	479	0.0	0.0	Lane 2	564	5.8	523	5.7	597 0.876	100	64.7	LOSE	26.8	196.6	Full	479	0.0	0.0
Lane 3	18	0.0	17	0.0	231 0.075	100	75.1	LOSE	0.8	5.4	Short	27	0.0	NA	Lane 3	18	0.0	17	0.0	233 0.072	100	74.5	LOSE	0.7	5.2	Short	27	0.0	NA
Approach	1159	5.3	1112 ^N	5.3	0.895		71.2	LOSE	29.7	217.3					Approach	1185	5.2	1100 ^N	5.2	0.876		65.5	LOSE	28.0	203.8				
Intersectio n	3963	7.9	3916 ^N	8.0	0.895		44.3	LOS D	35.4	262.7					Intersectio n	4012	7.7	3927 ^N	7.8	0.876		42.8	LOS D	37.5	277.7				



3.5.3 Mitigation 1

- Mitigation 1, to remedy the Ti Rakau Drive / Gossamer Drive intersection during the AM peak, includes the following on the northern Gossamer Drive approach (see figure below):
 - Adding an additional short right-turn lane (100 m).
 - o Converting the short left-turn slip lane to pass through the intersection and providing 150 m stacking space.
 - The kerbside short exit lane length is increased from 24 m to 100 m.

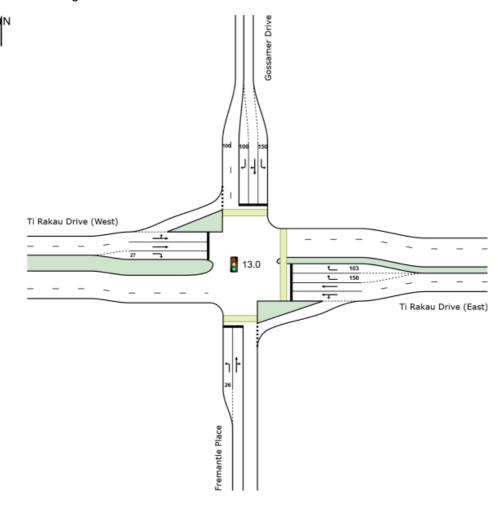


Figure 5: Ti Rakau Drive / Gossamer Drive - Mitigation 1 proposed layout



- The Stage 2 intersection performance (without mitigation) is expected to be poor (LOS F), especially in the Gossamer Drive right-turn lane into Ti Rakau Drive where delay increases from around 59 sec in the Do-Min scenario, to 320 sec (5.3 min).
- With the measures in place proposed under Mitigation 1, intersection performance during the detour is expected to be slightly improved, however still LOS F, with an average delay of 117 sec.
- Delay and queue lengths in the Gossamer Drive right-turn lanes are improved under Mitigation 1 (delay of 78 sec and average queue length of 46 m), compared to the Stage 2 detour.
- However, the additional control delay now imposed on the Gossamer Drive left-turn (previously a left-turn slip under the Do-Min and Stage 2 scenarios) has resulted in large delays (141 sec) and queues (478 m) in that lane. Traffic demand in this lane is around 960 veh/h.

Table 13: Ti Rakau Dr / Gossamer Dr - Stage 2 vs Mitigation 1 (AM)

Lane Use	and P	erforr	nance												Lane Use	and P	erforn	nance											
	DEM FLC	IAND)WS	ARR		Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config I	Lane Length	Cap. Adj.	Prob. Block.		DEM FLC		ARR FLC	IVAL DWS	Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config I	Lane Length	Cap. Adj.	Prob. Block
			[Total veh/h		veh/h v/c		sec		[Veh	Dist] m							HV] %			veh/h v/c					Dist] m				
South: Fre	mantle	Place													South: Free	mantle	Place												
Lane 1	23	8.7	23	8.7	63 0.367	100	94.2	LOSF	1.2	8.9	Short	26	0.0	NA	Lane 1	23	8.7	23	8.7	63 0.367	100	94.2	LOS F	1.2	8.9	Short	26	0.0	NA
Lane 2	28	7.1	28	7.1	65 0.429	100	92.7	LOSF	1.4	10.8	Full	285	0.0	0.0	Lane 2	28	7.1	28	7.1	65 0.429	100	92.7	LOS F	1.4	10.8	Full	285	0.0	0.0
Approach	51	7.8	51	7.8	0.429		93.4	LOSF	1.4	10.8					Approach	51	7.8	51	7.8	0.429		93.4	LOS F	1.4	10.8				
East: Ti Ra	kau Dri	ive (Ea	st)												East: Ti Ra	kau Dri	ve (Ea	st)											
Lane 1	832	10.3	832	10.3	680 1.224	100	272.4	LOSF	85.1	648.8	Full	636	0.0	51.9	Lane 1	847	10.4	847	10.4	778 1.088	100	158.3	LOS F	68.2	519.8	Full	636	0.0	31.2
Lane 2	792	10.5	792	10.5	646 1.224	100	277.1	LOSF	83.4	635.9	Full	636	0.0	50.0	Lane 2	777	10.5	777	10.5	714 ¹ 1.088	100	163.8	LOS F	64.8	494.0	Full	636	0.0	26.6
Lane 3	65	7.8	65	7.8	262 0.247	23	41.4	LOSD	1.8	13.7	Short	150	0.0	NA	Lane 3	126	7.8	126	7.8	598 0.210	57 ⁶	26.0	LOS C	2.6	19.4	Short	150	0.0	NA
Lane 4	282	7.8	282	7.8	262 1.075	100	137.8	LOSF	16.5	123.4	Short	103	0.0	NA	Lane 4	221	7.8	221	7.8	598 0.370	100	27.5	LOS C	4.9	36.6	Short	103	0.0	NA
Approach	1971	9.9	1971	9.9	1.224		247.4	LOSF	85.1	648.8					Approach	1971	9.9	1971	9.9	1.088		137.3	LOS F	68.2	519.8				
North: Gos	samer	Drive													North: Gos	samer	Drive												
Lane 1	960	9.0	960	9.0	1066 0.901	100	28.7	LOSC	30.7	231.8	Short	30	0.0	NA	Lane 1	960	9.0	960	9.0	884 ¹ 1.086	100	141.0	LOS F	63.4	478.0	Short	150	0.0	NA
Lane 2	264	6.4	264	6.4	212 ¹ 1.246	100	319.1	LOSF	29.7	219.6	Full	1010	0.0	0.0	Lane 2	132	5.9	132	5.9	235 0.562	100	77.6	LOS E	6.2	45.6	Full	1010	0.0	0.0
Approach	1224	8.4	1224	8.4	1.246		91.3	LOSF	30.7	231.8					Lane 3	131	6.3	131	6.3	233 0.562	100	77.9	LOS E	6.2	45.5	Short	100	0.0	NA
West: Ti Ra	akau Dr	rive (W	est)												Approach	1223	8.3	1223	8.3	1.086		127.4	LOS F	63.4	478.0				
Lane 1	523	11.8	432	11.6	451 0.958	100	98.1	LOSF	26.1	200.6	Full	479	0.0	0.0	West: Ti Ra	akau Dr	ive (We	est)											
Lane 2	500	11.9	413	11.7	431 0.958	100	96.7	LOSF	25.1	193.1	Full	479	0.0	0.0	Lane 1	522	11.8	431	11.6	549 0.786	100	57.6	LOS E	19.4	149 0	Full	479	0.0	0.0
Lane 3	10	10.0	8	9.6	102 0.081	100	85.7	LOSF	0.4	3.0	Short	27	0.0	NA	Lane 2	501	11.9	413	11.8	526 ¹ 0.786	100	57.3	LOSE	18.7	144.5	Full	479	0.0	0.0
Approach	1033	11.8	854 ^{N1}	11.7	0.958		97.3	LOSF	26.1	200.6					Lane 3	10	10.0	8	9.6	429 0.019	100		LOS D	0.3	2.3	Short	27	0.0	NA
Intersectio n	4279	9.9	4100 ^N	10.4	1.246		167.6	LOSF	85.1	648.8					Approach		11.8	_		0.786			LOSE	19.4	149.0				
n			1												Intersectio n	4278	9.9	4098 ^N	10.3	1.088		117.2	LOS F	68.2	519.8				

Therefore, alternative measures are proposed under Mitigation 2.



3.5.4 Mitigation 2

- Mitigation 2, to remedy the Ti Rakau Drive / Gossamer Drive intersection during the AM peak, includes the following on the northern Gossamer Drive approach (see figure below):
 - O Adding an additional short lane (100 m) for the shared through and right-turn movements.
 - Converting the centre lane to a full left-turn lane.
 - o Converting the short left-turn slip lane to pass through the intersection and adding 150 m stacking space.
 - The kerbside short exit lane length is increased from 24 m to 100 m.

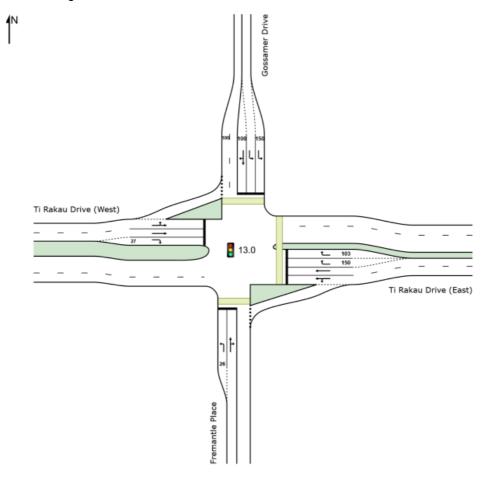


Figure 6: Ti Rakau Drive / Gossamer Drive - Mitigation 1 proposed layout



- The Stage 2 intersection performance (without mitigation) is expected to be poor (LOS F), especially in the Gossamer Drive right-turn lane into Ti Rakau Drive where delay increases from around 59 sec in the Do-Min scenario, to 320 sec (5.3 min).
- With the measures in place proposed under Mitigation 2, intersection performance during the detour is expected to be improved to LOS D and with an average delay of 37 sec. This is also an improvement compared to the Do-Min scenario (LOS D and delay of 48 sec).
- Delays and queue lengths in the Gossamer Drive right-turn lane are improved under Mitigation 2 (delay of 75 sec and average queue length of 86 m), compared to the Stage 2 detour.
- The Gossamer Drive left-turn lanes (previously a single left-turn slip under the Stage 2 scenario) are expected to experience improved performance (LOS C), with an average delay of 28 sec and average queue length of 91 m.

Table 14: Ti Rakau Dr / Gossamer Dr - Stage 2 vs Mitigation 2 (AM)

Lane Use	and P	erfor	mance												Lane Use	and P	erforn	nance											
	FLC		ARR FLC)WS	Deg. Cap. Satn	Lane Util.		Level of Service	BAC	RAGE K OF EUE	Lane Config	Lane Length	Cap. Adj.	Prob. Block.		DEM FLC		ARR FLO		Deg. Cap. Satn	Lane Util.		Level of Service	AVER BAC QUI	K OF	Lane Config I	Lane Length	Cap. Adj.	Prob. Block.
			[Total veh/h		veh/h v/c		sec		[Veh	Dist] m						[Total veh/h	HV] %	[Total veh/h	HV] %	veh/h v/c	%	sec		[Veh	Dist] m			%	%
South: Fremantle Place							South: Free	mantle	Place																				
Lane 1	23	8.7	23	8.7	63 0.367	100	94.2	LOSF	1.2	8.9	Short	26	0.0	NA	Lane 1	23	8.7	23	8.7	79 0.289	100	73.8	LOS E	0.9	7.0	Short	26	0.0	NA
Lane 2	28	7.1	28	7.1	65 0.429	100	92.7	LOSF	1.4	10.8	Full	285	0.0	0.0	Lane 2	28	7.1	28	7.1	83 0.338	100	72.2	LOS E	1.1	8.4	Full	285	0.0	0.0
Approach	51	7.8	51	7.8	0.429		93.4	LOSF	1.4	10.8					Approach	51	7.8	51	7.8	0.338		72.9	LOS E	1.1	8.4				
East: Ti Rakau Drive (East)										East: Ti Rakau Drive (East)																			
Lane 1	832	10.3	832	10.3	680 1.224	100	272,4	LOSF	85.1	648.8	Full	636	0.0	51.9	Lane 1	846	10.4	846	10.4	960 0.881	100	36.2	LOS D	31.8	242.5	Full	636	0.0	0.0
Lane 2	792	10.5	792	10.5	646 1.224	100	277.1	LOSF	83.4	635.9	Full	636	0.0	50.0	Lane 2	778	10.5	778	10.5	883 ¹ 0.881	100	36.1	LOS D	28.5	217.7	Full	636	0.0	0.0
Lane 3	65	7.8	65	7.8	262 0.247	236	41.4	LOSD	1.8	13.7	Short	150	0.0	NA	Lane 3	126	7.8	126	7.8	320 0.393	57 ⁶	31.4	LOS C	2.5	18.8	Short	150	0.0	NA
Lane 4	282	7.8	282	7.8	262 1.075	100	137.8	LOSF	16.5	123.4	Short	103	0.0	NA	Lane 4	221	7.8	221	7.8	320 0.692	100	33.9	LOS C	4.8	36.0	Short	103	0.0	NA
Approach	1971	9.9	1971	9.9	1.224		247.4	LOSF	85.1	648.8					Approach	1971	9.9	1971	9.9	0.881		35.6	LOS D	31.8	242.5				
North: Gos	samer	Drive													North: Gos	samer	Drive												
Lane 1	960	9.0	960	9.0	1066 0.901	100	28.7	LOSC	30.7	231.8	Short	30	0.0	NA	Lane 1	480	9.0	480	9.0	687 0.698	100	27.4	LOS C	12.0	90.2	Short	150	0.0	NA
Lane 2	264	6.4	264	6.4	212 ¹ 1.246	100	319.1	LOSF	29.7	219.6	Full	1010	0.0	0.0	Lane 2	480	9.0	480	9.0	687 0.698	100	27.4	LOS C	12.0	90.2	Full	1010	0.0	0.0
Approach	1224	8.4	1224	8.4	1.246		91.3	LOSF	30.7	231.8					Lane 3	264	6.4	264	6.4	296 0.891	100	74.7	LOS E	11.6	85.9	Short	100	0.0	NA
West: Ti Ra	akau Di	rive (W	est)												Approach	1224	8.4	1224	8.4	0.891		37.6	LOS D	12.0	90.2				
Lane 1	523	11.8	432	11.6	451 0.958	100	98.1	LOSF	26.1	200.6	Full	479	0.0	0.0	West: Ti Ra	akau Dr	ive (We	est)											
Lane 2	500	11.9	413	11.7	431 0.958	100	96.7	LOSF	25.1	193.1	Full	479	0.0	0.0	Lane 1	523	11.8	431	11.6	669 0.645	100	37.1	LOS D	13.6	104.3	Full	479	0.0	0.0
Lane 3	10	10.0	8	9.6	102 0.081	100	85.7	LOSF	0.4	3.0	Short	27	0.0	NA	Lane 2	500	11.9	413	11.8	640 ¹ 0.645	100	36.2	LOS D	13.1	101.3	Full	479	0.0	0.0
Approach	1033	11.8	854 ^{N1}	11.7	0.958		97.3	LOSF	26.1	200.6					Lane 3	10	10.0	8	9.6	117 0.071	100	67.7	LOS E	0.3	2.3	Short	27	0.0	NA
Intersectio	4279				1.246		167.6	LOSF	85.1	648.8					Approach	1033	11.8	852 ^{N1}	11.7	0.645		37.0	LOS D	13.6	104.3				
n	4213	3.3	1	10.4	1.240		107.0	2001	03.1	040.0					Intersectio	4279	9.9	4098 ^N	10.4	0.891		36.9	LOS D	31.8	242.5				

• <u>Mitigation 2 is expected to result in improved performance compared to Mitigation 1 and it is recommended that this mitigation measure be discussed with key stakeholders.</u>

4.0 Traffic Volumes in the wider Project Area

- **Table 15** below shows an increase in demand in the Gossamer Drive left-turn at the intersection with Pakuranga Road, and in the Gossamer Drive right-turn lane at Ti Rakau Drive during the AM and PM.
- The table below also shows an increase in the Ti Rakau Drive west approach left-turn lane into Gossamer Drive during the PM.
- Overall, it seems that only a small percentage of traffic is routing along the proposed detour route (probably due to the already congested nature of those intersections).
- As a result, in the **inbound** (citybound) direction during the AM demand seems to detour via Gossamer Road to Pakuranga Road in the north and Ti Rakau Drive in the south.
- In the PM in the **outbound** direction, demand seems to return via Pakuranga Road, and via SEART turning right onto Ti Rakau Drive.

Table 15: Gossamer Road traffic volumes

				OM AM peal	k 0645-074	Stage 2 AM peak 0800-0900					
			Traffic Flow (Vehicles / hr)			hr)	Tr	(Vehicles /	/ hr)		
			Car	Truck	Bus	Total	Car	Truck	Bus	Tota	
	B.1	Through	503	28	6	536	613	40	6	659	
	Pakuranga Road (West)	Right	143	13	0	156	139	19	0	158	
		Left	847	20	3	871	839	32	3	874	
Pakuranga Road /	Pakuranga Road (East)	Through Bus-lane	0	0	12	12	0	0	10	10	
Gossamer Drive	, , , , , , , , , , , , , , , , , , , ,	Through	1633	83	0	1715	1022	50	0	108	
		Left	59	5	0	63	100	7	0	10	
	Gossamer Drive	Right	273	10	2	285	253	13	2	26	
	Total (Maximum in Red)	rugin	3457	159	23	3639	2975	162	21	315	
	Total (mariman in 1000)	Left	18	1	0	19	34	4	0	37	
	Ti Rakau Drive (North)	Through	733	71	14	817	868	104	13	98	
	Trakad Dilve (NOItil)	Right	9	1	0	10	9	1	0	10	
		Left	974	79	0	1053	874	86	0	96	
	Gossamer Drive	Through		1	0	10		. 1	0	10	
Ti Rakau Drive /	Coodanier Brito	Right	83	7	0	90	238	16	0	25	
Gossamer Drive /		Left	13	1	0	14	18	1	0	18	
Fremantle Place	Ti Rakau Drive (South)	Through	1490	173	16	1679	1437	152	16	160	
Tremande Flace	Tirkakaa biive (Soaiii)	Right	229	21	0	250	320	27	0	34	
		Left	24	1	0	25	21	2	0	2:	
	Fremantle Place		13	0	0	13	10	0	0	10	
	I Telliantie Place	Through	17	1	0	18	16	2	0	18	
	Total (Massianus in Dad)	Right	3602	357	29	3988	3842	396	29	426	
	Total (Maximum in Red)		3002	331	25	3300	3042	350	25	421	
				OM PM peal	1615-171	Stage 2 PM peak 1615-1715					
				affic Flow		Traffic Flow (Vehicles / hr)					
			Car	Truck	Bus	Total	Car	Truck	Bus	Tot	
		Through	1920	84	14	2017	1795	79	14	188	
	Pakuranga Road (West)	Right	44	2	0	46	47	5	0	5	
		Left	248	7	2	257	232	7	2	24	
Pakuranga Road /	Pakuranga Road (East)	Through Bus-lane	0	0	6	6	0	0	6	6	
Gossamer Drive	rakulaliya Road (Last)	Through bus-lane	1104	45	0	1149	1107	45	0	119	
Gossaillei Diive		Left	4	6	0	10	4	6	0	10	
	Gossamer Drive		593	14	4	611	595	14	4	61	
	Total (Maximum in Red)	Right	2011	158	26	4095	3770	155	26	396	
	Total (Maximum in Red)	Left	59	2	0	61	126	2	0	12	
	Ti Rakau Drive (North)		1021	42	17	1079	979	43	17	103	
	ii Rakau Drive (North)	Through	1021	0	0	1079	18	0	0	10.	
		Right	393	82	0	475	382	73	0	45	
	Conners Drive	Left	393	1	0	10	8	2	0		
Ti Daleau Daine 1	Gossamer Drive	Through			_		_		_	10	
Ti Rakau Drive /		Right	30 20	0	0	31 20	40 20	0	0	41	
Gossamer Drive /	Ti Balana Bahas (C. 11)	Left		_	_			_	_	2	
Fremantle Place	Ti Rakau Drive (South)	Through	1508	101	13	1622	1556	105	13	167	
		Right	553	54	0	606	540	50	0	59	
		Left	12	0	0	12	10	0	0	10	
	Fremantle Place	Through	10	0	0	10	10	0	0	10	
		Right	16	1	0	18	16	1	0	18	
	Total (Maximum in Red)		3640	285	30	3956	3700	278	30	400	



5.0 Impacts to Buses

- The impacts to buses through the project area are expected to be low as the frequency of the 711 route in the AM inbound direction is only 4 buses/h (1 every 15min).
- As shown above, the William Roberts Road / Reeves Road intersection is expected to operate with good LOS in the AM.
- The William Roberts Road / Pakuranga Road intersection is already saturated in the Do-Min AM scenario and the impact of the detour is expected to be negligible.
- The Pakuranga Road / Ti Rakau Drive intersection is expected to operate at LOS D during the AM.



Figure 7: Inbound buses through project area



- NOTE: The 711 outbound route below is based on the Do-Min scenario. During Stage 2, with Reeves Road closed between Ti Rakau Drive and William Roberts Road, it is anticipated buses will continue along Ti Rakau Drive and turn left at the newly completed Ti Rakau Drive / William Roberts Road intersection.
- The impacts to buses through the project area are expected to be low as the frequency of the 711 route in the PM outbound direction is only 4 buses/h (1 every 15min).
- The Pakuranga Road / Ti Rakau Drive intersection is expected to perform at LOS F during the Stage 2 PM scenario, compared to the LOS E of the Do-Min scenario, however, the Pakuranga Road west approach is already at capacity (including the bus lanes).
 - However, performance is expected to improve at this intersection with the mitigation measures proposed under Section 3.4.
- The Ti Rakau Drive / SEART intersection is already saturated during the PM peak and the impact of the detour is expected to be negligible.
 - However, performance is expected to improve at this intersection with the mitigation measures proposed under Section 3.4.
- It is expected that the Ti Rakau Drive / William Roberts Road LILO intersection would operate with acceptable LOS and the impact of the detour would be negligible at this intersection.

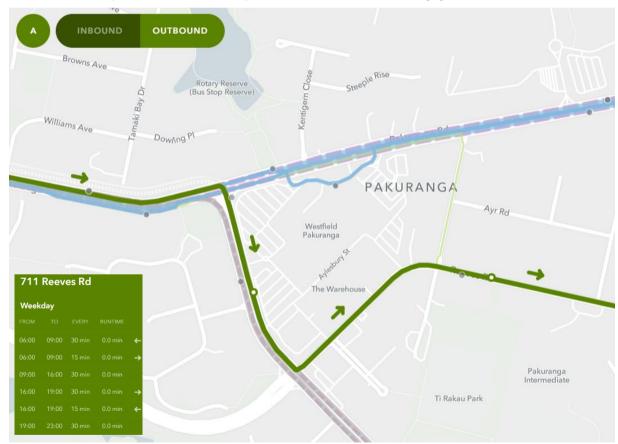


Figure 8: Outbound buses through project area